DEPARTEMENT SAFETY STRUCTURES FIRE

Mechanical and Fire Resistance Division

REPORT No MRF 1526054277/B

On
Fire Evaluation of
Post installed rebar connections
With HILTI HIT-RE 500 V3 injection system

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It comprises 75 pages numbered from 1/75 to 75/75

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1. TOPIC

When subjected to fire exposure, construction elements performances are reduced by the effect of the temperature increase. At the HILTI company request, CSTB has performed a study aimed at the evaluation of the fire behaviour of the HILTI – RE 500 V3 injection resin system used in conjunction with concrete reinforcing rebar (d 8 to 32 mm).

The present study is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This report presents values of bond capacities and load capacities respectively an for overlap joint application and for an anchorage application using the mortar product HILTI – RE 500 V3.

WARNING

This report does not deal with the mechanical design at ambient temperature; neither does it deal with the design according to other accidental solicitations. Design at ambient temperature shall be carried out before fire design.

2. REFERENCES

[1] EAD 330087-00-0601, SYSTEMS FOR POST-INSTALLED REBAR CONNECTIONS WITH MORTAR, Draft April 2015

[2] TEST REPORT No MRF 1526054277, Fire Testing of Post installed rebar connections with HILTI – RE 500 V3 injection mortar, 2015, Centre Scientifique et Technique du Bâtiment

[3] CEN. EN 1991-1-2. Eurocode 1, Part 1-2: Actions on structures: general actions – actions on the structures exposed to fire. CEN, Bruxelles, Belgique; 2002.

[4] CEN. EN 1992-1-1. Eurocode 2, Part 1-1: Design of concrete structures - General rules and rules for buildings. CEN, Bruxelles, Belgique; 2005.

[5] CEN. EN 1992-1-2. Eurocode 2, Part 1-2: Design of concrete structures – General rules and structural fire design. CEN, Bruxelles, Belgique; 2005.

3. AUTHORS

Marne-la-Vallée, FRANCE,
on 12th of April 2016

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4. BACKROUND

4.1 Evaluation method

The fire evaluation is performed with three steps.

1) First, an experimental program of pullout tests at high temperatures is carried out in order to determine a relationship between bond resistance and temperature [2]. This relationship is then expressed by a temperature reduction factor $0<k(\theta)<1$ which describes the decrease of resistance of the bond system (see PART 5).
2) Secondly, a thermal calculation using the method described in EN 1991-1-2, section 3 [3] is performed in order to determine the temperature distribution along the bonded rebar for each fire duration and for a given structural configuration.
3) Finally, at each time during the fire, the bond resistances are determined along the bonded rebar. For the anchor application the load resistance is calculated by integrating the bond resistances along the embedded depth.

Figure 1 presents the steps of the method used in this evaluation and the corresponding parts of the report.

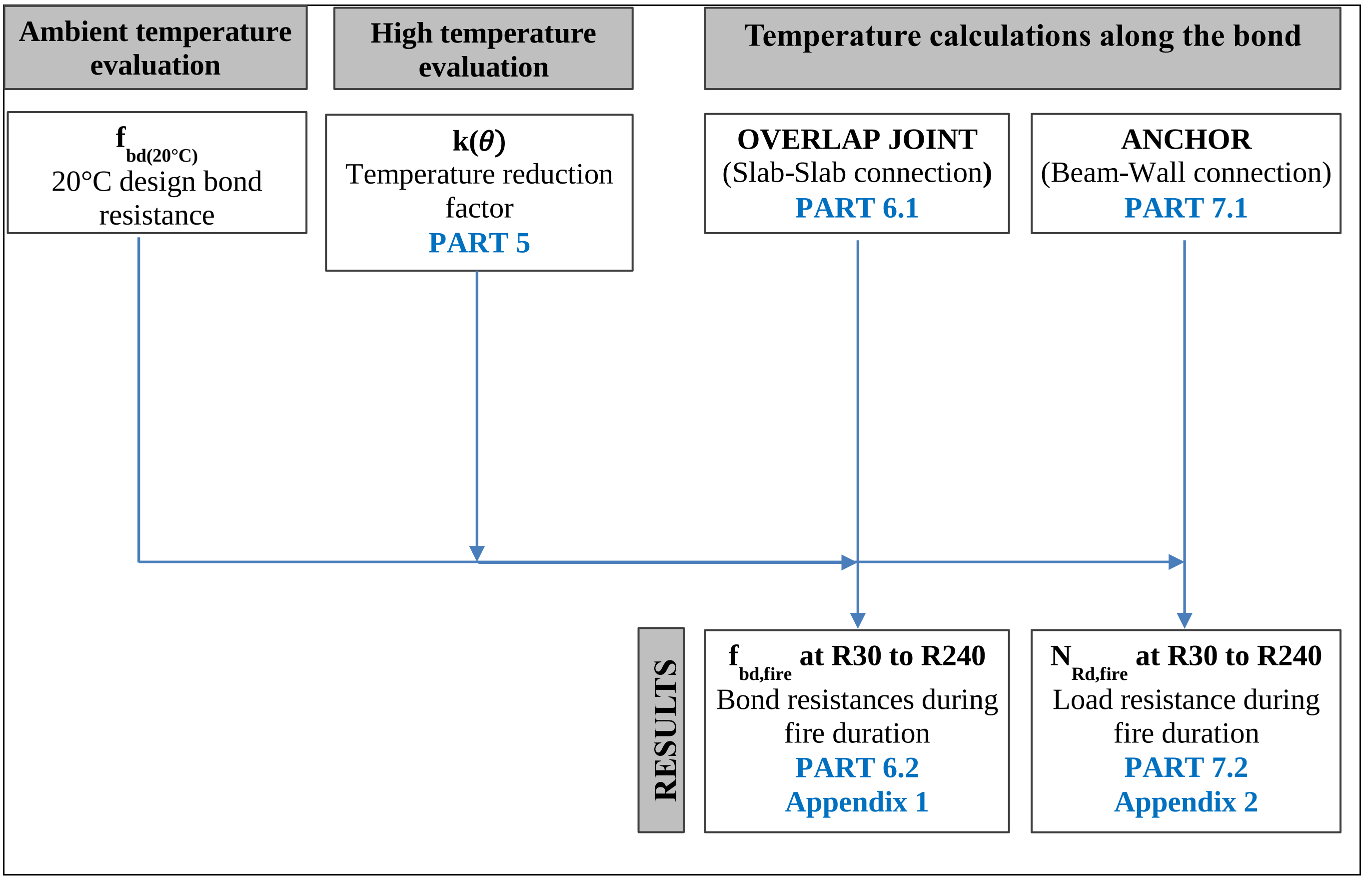


Figure 1 : Method used for fire evaluation of bonded rebars

The evaluation covers two structural uses of post-installed rebars in concrete (Figure 2): i) the overlap joint application and ii) the anchor application.

i) In the overlap joint application for a slab-slab configuration where the lower surface is exposed to fire, the temperature is uniform. The bond resistance is uniform along the bond and depends on the concrete cover and the duration of the fire (PART 6.2).

ii) In the anchor application for a beam-wall configuration where at least one side of the wall is exposed to fire, the temperature along the bond (inside the wall) is not uniform. This leads to different bond resistances and the load resistance is calculated by integration of the bond resistances along the lateral surface of the rebar (PART 7.2).

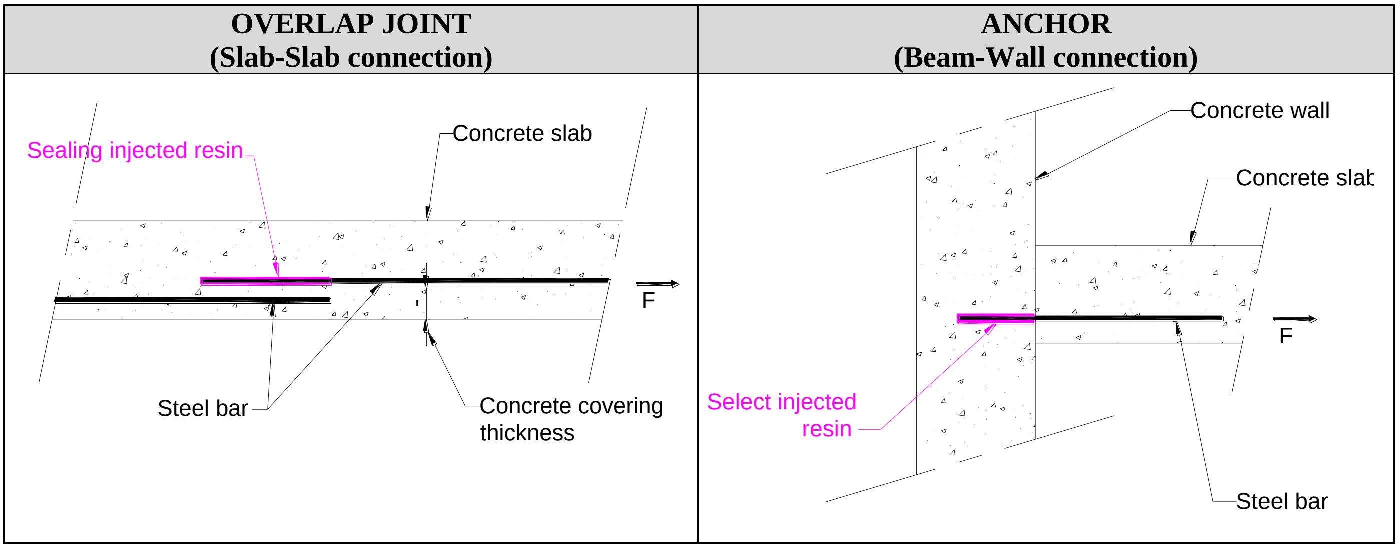


Figure 2 : Sketches of a Slab-Slab connection left and of a Beam-Wall connection

4.2 Application scope

The values of load resistances presented in this report are applicable for given parameters: Concrete class, structural configuration, fire duration, bar diameter, bond length, concrete cover and maximal temperatures. The result tables are provided in appendices 1 and 2.

i) **Concrete class**
The fire evaluation is applicable for C20/25 and higher resistance classes of concrete. According to the EAD [1], the ultimate bond resistance in C20/25 concrete is equal to f_{bd}=2,30 N/mm² for bar diameters between 8 and 32 mm.

ii) **Structural configurations**
The fire evaluation covers slab-slab and beam-wall configurations for beams with a width higher than 40 cm. Load resistances of the beam-wall configuration may be conservatively applied to a slab-slab configuration. The bond resistances of the slab-slab configuration SHALL NOT be applied to a beam-beam configuration.

iii) **Fire durations**
The bond resistances and load resistances are provided at 30, 60, 90, 120, 180 and 240 min under a standardized ISO 834-1 fire. Thermal loading is calculated using the method described in EN 1991-1-2, section 3 [3].

iv) **Bar diameters**
The fire evaluation covers steel rebars with diameters of 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm with a yield strength of 500 N/mm².

v) Bond lengths

For the slab-slab configuration, the bond resistances are provided. The calculation of the bond length shall be carried out in accordance with EN 1992-1-1, section 8 [4].

For the beam-wall connection, the load capacities are calculated for lengths between the minimal length $l_{b,min}$ and the maximal anchorage length conditioned by the yielding of steel. The minimal embedment length $l_{b,min}$ is calculated in accordance with EN 1992-1-1, section 8 [4] (see equation below). The evaluation is applicable for steel reinforcement bars presenting a tensile resistance equal or higher than 500 N/mm².

$l_{fire,min} = l_{b,min} = \max\{0{,}3.l_{b,rqd}\ ;\ 10.d\ ;\ 100\ \text{mm}\}$

Where $l_{b,rqd}$ is the required basic anchorage length

$$l_{b,rqd} = \frac{d}{4}.\frac{\sigma_{sd}}{f_{bd}} = \frac{d}{4}.\frac{\sigma_{s,yeild}}{\gamma_M.f_{bd}}$$

Where:

$\sigma_{s,\ yield}$ = 500 N/mm² is the yield stress of steel
γ_M = 1,5 is the material coefficient
f_{bd} = 2.3 N/mm² is the design bond strength in C20/25 concrete.
d is the diameter of the bar

$$N_{rebar\ yeild} = \frac{\sigma_{s,yeild}}{\gamma_{M,20°C}}.\pi.\left(\frac{d}{2}\right)^2$$

Where:

$\sigma_{s,\ yield}$ = 500 N/mm² is the yield stress of steel
$N_{rebar\ yield}$ is the design yielding load of the rebar
γ_M = 1,5 is the material coefficient
d is the diameter of the bar

Table 1 presents the minimal embedment lengths and yielding loads.

Table 1 : Minimal embedment lengths and yielding loads

Rebar diameter (mm)	**8**	**10**	**12**	**14**	**16**	**20**	**25**	**28**	**32**
Required anchorage length $l_{b,rqd}$ (mm)	290	362	435	507	580	725	906	1014	1159
Minimum anchorage length $l_{b,min}$ (mm)	100	109	130	152	174	217	272	304	348
Design Yielding load of the rebar (kN)	16.8	26.2	37.7	51.3	67.0	104.7	163.6	205.3	268.1

vi) Concrete cover

Choice of the concrete cover shall be carried out in accordance with EN 1992-1-1, section 4 [4]. In this evaluation, concrete cover is only considered for the thermal protection it brings to the rebar.

For the slab-slab configuration, bond resistances are provided for different concrete covers starting at 40 mm.
For the beam-wall connection, the concrete cover in the beam influences the temperature distribution along the rebar in the thickness of the wall. The load resistances are provided for concrete covers inside the beam of 10, 20, 30 and 40 mm. Results are only provided for concrete covers superior to the diameter of the bar in accordance with EN 1992-1-1, section 4 [4].

vii) Maximal temperatures

In accordance to EN 1992-1-2, section 5 [5] steel resistance remains constant between 20°C and 350°C for bar laminated at high temperature. Therefore resistances are only considered along the parts of the bond below 350°C. Furthermore, the resistance is considered equal to zero above the temperature θ_{max} (described in PART 5.1) linked to the mortar behavior.

5. TEMPERATURE REDUCTION FACTOR

From the pull-out tests [2] a power trend curve is used to describe the bond resistance-temperature relationship analytically between 20°C and 152°C using the fallowing equation.

$$f_{bm}(\theta) = a.\theta^{-b.}$$

Where:

$f_{bm}(\theta)$ *is the* mean bond resistance at the temperature θ (in N/mm²)
θ is the temperature of the bond material
a and b are the fitting curve constants

The maximal temperature reached during the tests is identified as θ_{max}.

For the HILTI – RE 500 V3 injection system, the a, b and θ_{max} parameters are presented in Table 3.

Table 3 : Injection system parameters

Mortar parameters		
a=	1178.2	N/mm²
b=	1.255	/°C
θ max=	152	°C
θ1=	45	°C

The temperature reduction factor k(θ) is determined from the fitted curve $f_{bm}(\theta)$ to describe the variation of resistance of the injection system with temperature. It is calculated using the fallowing equations.

$$k(\theta) = \frac{f_{bm}(\theta)}{f_{bm,req,d}} \leq 1 \quad for \quad 20°C \leq \theta \leq \theta_{max}$$

$$k(\theta) = 0 \quad for \quad \theta > \theta_{max}$$

Where:

$k(\theta)$ temperature reduction factor
$f_{bm}(\theta)$ *is* the mean bond resistance at the temperature θ
$f_{bm,req,d}$ =*min{10 N/mm² ;* $f_{bm}(\theta)$*} is the* required bond resistance at cold state
θ *is the* temperature of the bond
θ_{max} maximal temperature measured during the tests

Figure 4 presents the variation of the temperature reduction factor vs. temperature for the HILTI – RE 500 V3 injection system.
No extrapolation beyond test temperatures is allowed. For temperatures higher than the maximal measured temperature during the tests (θ_{max}), the reduction factor k(θ) is equal to zero.

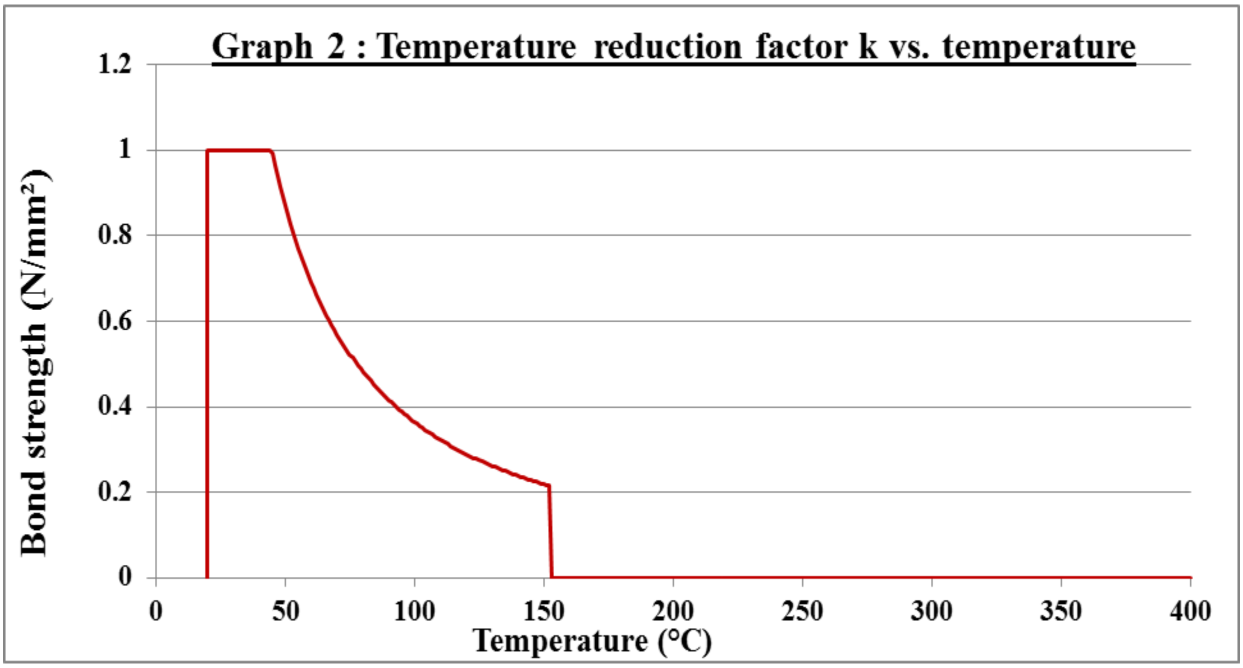


Figure 4 : Temperature reduction factor k(θ) vs. temperature

6. OVERLAP JOINT APPLICATION (SLAB-SLAB CONNECTION)

6.1 Temperature fields

The knowledge of the fire behaviour of traditional concrete structures to assess the temperature distribution for every fire duration by modeling the thermal exchanges inside concrete elements. The temperature profile depends on the connection configuration: slab-slab or beam-wall. These temperatures are calculated using the finite elements method in accordance with EN 1991-1-2, section 3 [3] with the CAST3M software.

At the initial time (t=0) every element temperature is supposed equal to 20°C.

The fire is modeled by a heat flux on the exposed faces of the structure. This heat flux is a function of the gas temperature θ_g for which the evolution is given by the conventional ISO 834-1 time-temperature relationship (EN 1991-1-2, section 3 [3]).

$$\boldsymbol{\theta_g(t) = \theta_0 + 345.log_{10}(8.t + 1)}$$

Where:

θ_q is the gas temperature
θ_0=20°C is the initial temperature
t is the time in minutes

The entering flux in a heated element is the sum of the convective and the radiation parts:

- convective flux density: $\boldsymbol{\varphi_c = h.(\theta_g - \theta_s)}$ (W/m^2),
- radiation flux density: $\boldsymbol{\varphi_c = \varepsilon.\sigma.\left(\theta_g^4 - \theta_s^4\right)}$ (W/m^2).

Where:

σ is the Stefan-Boltzmann parameter
θ_s is the surface temperature of the heated element
ε is the resulting emissive coefficient
h is the exchange coefficient for convection

The exchange coefficients, presented in Table 4, are given by EN 1992-1-2, appendix A [5].

Table 4 : Values for the exchange coefficients

	h(W/m²K)	ε
Fire exposed side	**25**	***0.7***

In this study, only concrete is considered in the thermal calculation (EN 1992-1-2, section 4 [5]). The concrete thermal properties are provided by EN 1992-1-2, section 3 [5]. The variations of thermal conductivity, mass density and specific heat are represented in Figure 5. The peak of the specific heat corresponds to a concrete having a water percentage of 1,5% in accordance with EN 1992-1-2, appendix A [5].

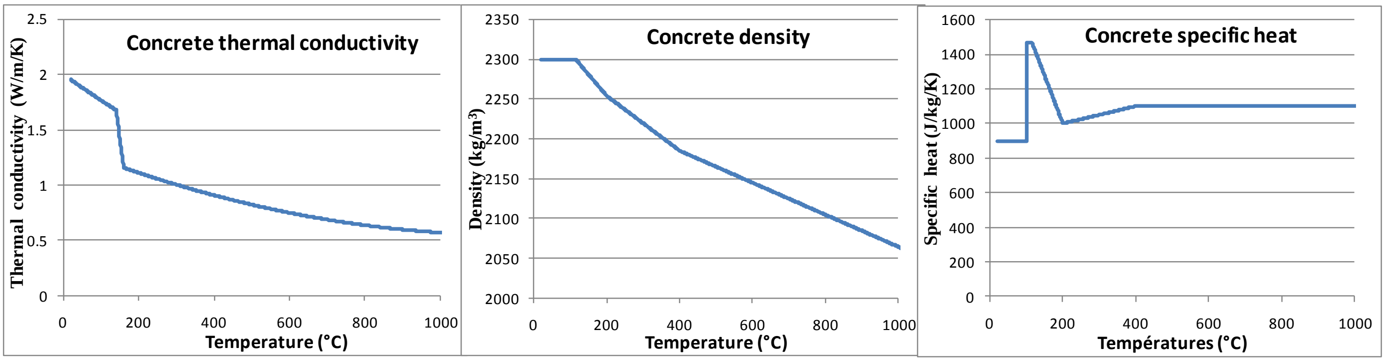


Figure 5 : Variations of thermal conductivity, density and specific heat of concrete according to EN 1992-1-2

For a slab-slab connection (Figure 2), the thermal calculation is carried out on a two dimensional mesh by applying the fire heat flux as boundary condition on the lower surface. No boundary condition at 20°C is applied on the upper surface to be conservative.

The isotherms are horizontal implying that the temperature is uniform along the bonding interface and equal to the temperature in a slab at a depth equivalent to the concrete cover. Figure 6 presents the temperature versus concrete cover at 0, 30, 60, 90, 120, 180 and 240 min during an ISO 834-1 fire. The same temperature curves are provided in EN 1992-1-2, appendix A [5].

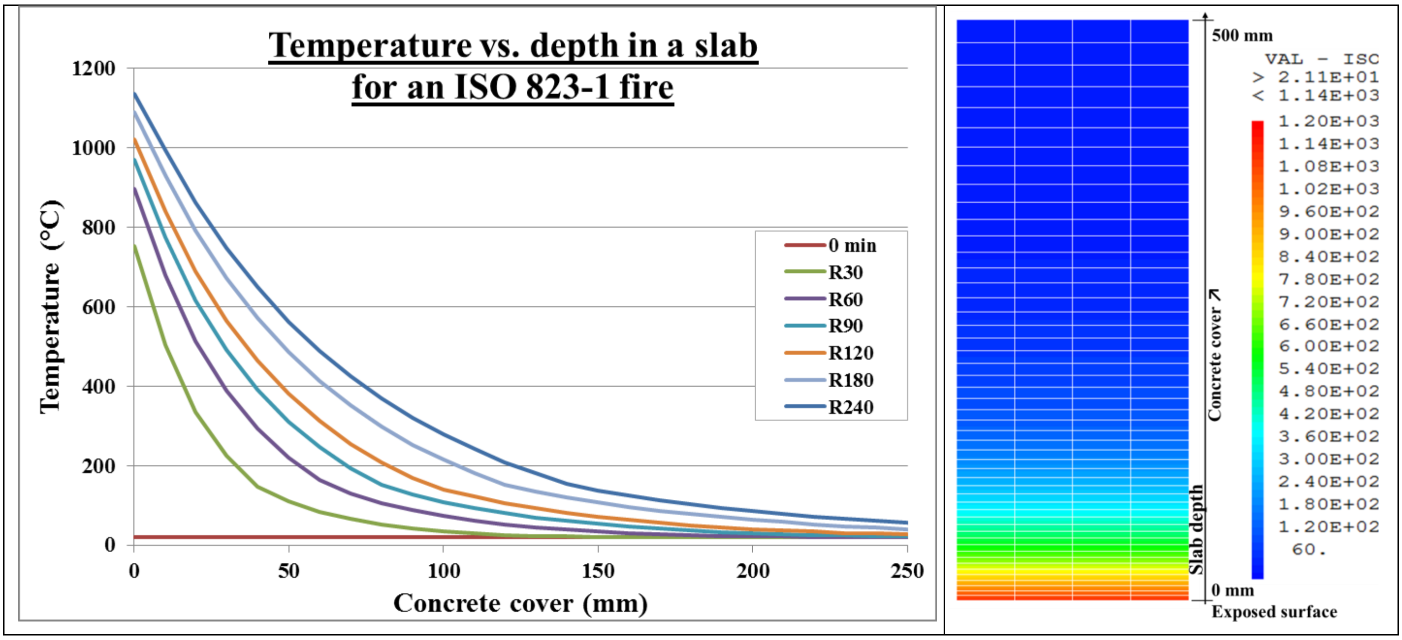


Figure 6 : Temperature vs. concrete cover temperature at 0, 30, 60, 90, 120, 180 and 240 min during an ISO 834-1 fire

6.2 Design bond resistances

From the temperature curves (Part 6.1, Figure 6) and the temperature reduction factor k(θ) (Part 5.2, Figure 4), the values of the design bond resistances $f_{bd,fire}$ are determined using the fallowing equation.

$$f_{bd,fire}(\theta) = f_{bd,20^\circ C}\cdot\frac{\gamma_{M,20^\circ C}}{\gamma_{M,fire}}.k(\theta)$$

Where:

$f_{bd,fire}(\theta)$ is the design bond resistance that depends on temperature
$f_{bd,20^\circ C}=2{,}3$ for C20/25 concrete is the design bond strength at 20°C
$\gamma_{M,20^\circ C}=1{,}5$ is the material coefficient at ambient temperature
$\gamma_{M,fire}=1$ is the material coefficient in a fire situation
k(θ) is the temperature reduction factor

Appendix 1 presents values of the design bond resistance for different concrete covers at 30, 60, 90, 120, 180 and 240 min during an ISO 834-1 fire.

The material safety factor applicable for the accidental situation of fire is equal to 1 according to EN 1992-1-2, section 2 [5], while it is equal to 1,5 at ambient temperature. This leads to obtaining higher values of load resistances at the beginning of a fire in fire design in comparison to ambient temperature design for the same rebar geometry. Design at ambient temperature shall be carried out before fire design.

7. ANCHOR APPLICATION (BEAM-WALL CONNECTION)

7.1 Temperature fields

For a beam-wall connection (Figure 2) where the rebar is bonded inside the wall, there is a temperature gradient in the thickness of the wall. The temperature along the bonding interface is not uniform and depends on the fire duration, the anchoring length and the concrete cover of the rebar inside the beam (which acts as a protection against thermal exposure). Therefore, the temperature profiles along the bond are determined for each fire duration, for each bonded length and for the concrete covers inside the beam of 10, 20, 30 and 40 mm.

A three dimensional mesh was used. Due to symmetry considerations only half of the structure is meshed (Figure 7). The same calculation parameters (material thermal properties, time-temperature curve, convective and radiation exchange coefficients) as the ones described in PART 6.1 are applied.

The boundary conditions are:

- On the lower and lateral sides of the beam fire heat fluxes are applied to the elements.
- On the side of the wall where the beam is connected, the fire heat fluxes are applied to the elements.
- No heat exchange condition is applied on the other sides.

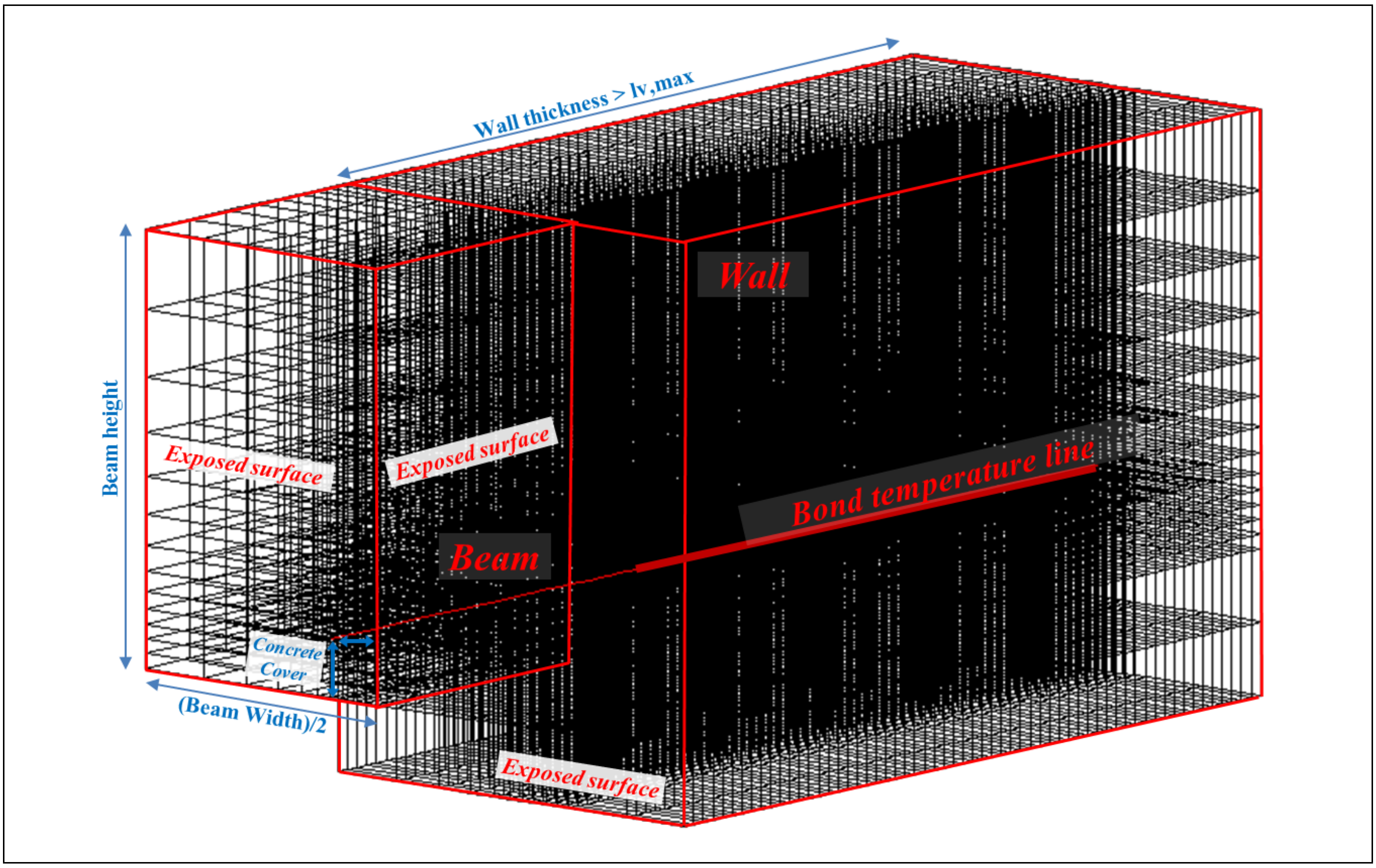


Figure 7 : Mesh used for thermal calculations for the beam-wall connection

Figure 8 presents the calculated thermal fields at 30, 90 and 240 min. The geometry of the mesh of the beam used for calculations is taken large enough so that the isotherms at 240 min of heating are parallel to the concrete surfaces (Figure 8). This implies that the same temperature profiles along the rebar would be obtained for larger and higher beams. The beam height was equal to 300 mm and the beam width was equal to 400 mm.

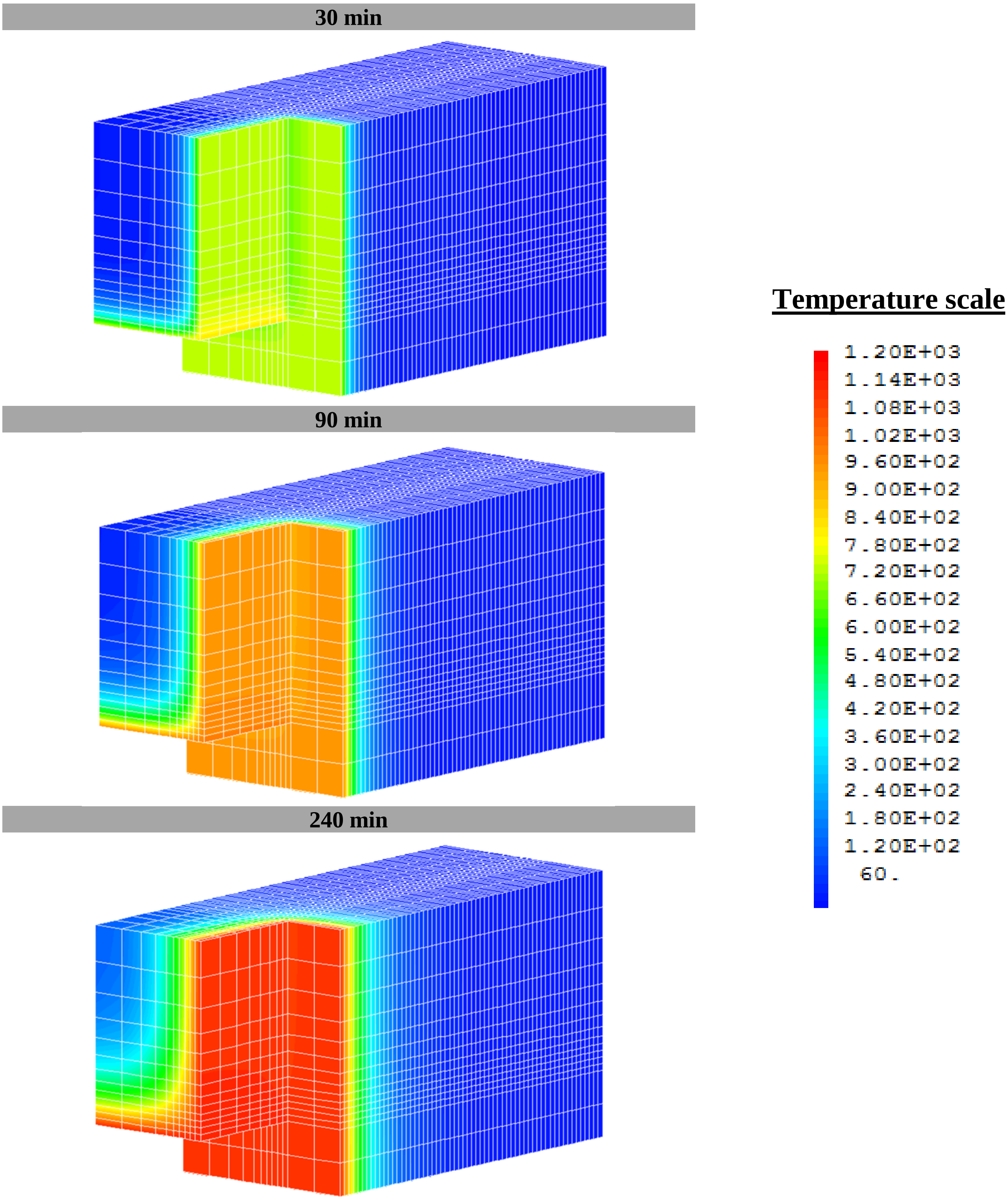


Figure 8 : Temperature fields at 30, 90 and 240 min during an ISO 834-1 fire for the beam-wall connection

7.2 Design load resistances

From the calculated temperature profiles and from the temperature reduction factor k(θ) (Part 5.2, Figure 4), the values of design load capacities $N_{Rd,fire}$ are determined by integration of the design bond resistances.

$$N_{Rd,fire} = \pi.d.\int_0^{l_v} f_{bd,fire}\big(\theta(x)\big).dx = \pi.d.f_{bd,20°C}.\frac{\gamma_{M,20°C}}{\gamma_{M,fire}}.\int_0^{l_v} k\big(\theta(x)\big).dx$$

Where:

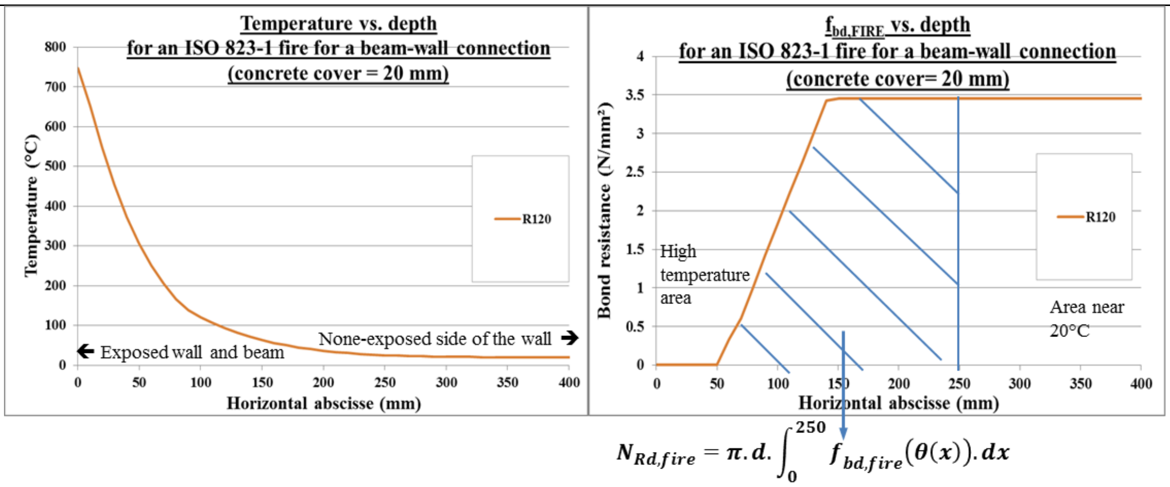
$N_{Rd,fire}$ is the design load resistance at a given time during the fire
$f_{bd,20°C}$=2,3 for C20/25 concrete is the design bond strength at 20°C
$\gamma_{M,20°C}$=1,5 is the material coefficient at ambient temperature
$\gamma_{M,fire}$=1 is the material coefficient in a fire situation
k(θ) is the temperature reduction factor
l_v is the embedment depth of the bonded rebar

The integration is performed by finite differences using the following equation.

$$N_{Rd,fire} \approx \pi.d.f_{bd,20°C}.\frac{\gamma_{M,20°C}}{\gamma_{M,fire}}.\sum_0^{l_v} k(\theta_i).\Delta x$$

For the calculation, the value of Δx was taken equal to 10 mm and the maximal temperature reduction factor k(θ_i) on the length of Δx was taken into account.

Figure 9 presents a general example (not from the HILTI – RE 500 V3 mortar) of the calculation of the design load resistance by integration of f_{bd} on a bond length of 250 mm by using the temperature profile along the bond at 120 min during an ISO 834-1 fire with a concrete cover of 20 mm in the beam.



$$N_{Rd,fire} = \pi.d.\int_0^{250} f_{bd,fire}\big(\theta(x)\big).dx$$

Figure 9 : General example of the calculation of the design load resistance by integration of f_{bd}

Appendices 2.1, 2.2, 2.3 and 2.4 present the values of $N_{Rd,fire}$ at different fire durations for different bond lengths respectively for concrete covers of 10 mm, 20 mm, 30 mm and 40 mm. The minimal and maximal values of bond lengths are in accordance with PART 4.2.

8. LIST OF APPENDICES

Appendix 1: Design bond resistances for an overlap joint application (slab-slab connection)

Appendix 2.1: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 10 mm for diameters 8 and 10 mm

Appendix 2.2: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 20 mm for diameters 8, 10, 12, 14, 16 and 20 mm

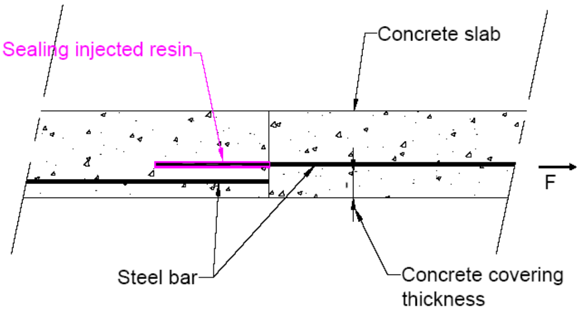
Appendix 2.3: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 30 mm for diameters 8, 10, 12, 14, 16, 20, 25 and 28 mm

Appendix 2.4: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 40 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

Appendix 1:
Maximum applicable bond stress for an overlap joint application

The table presents design bond resistances (f_{bd}) for a **Slab-Slab connection** using **C20/25 concrete** and rebars with a yield strength **f_y=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for concrete covers between 30 and 230 mm.

The bond resistance values shall not be applied for beam-beam connections. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.



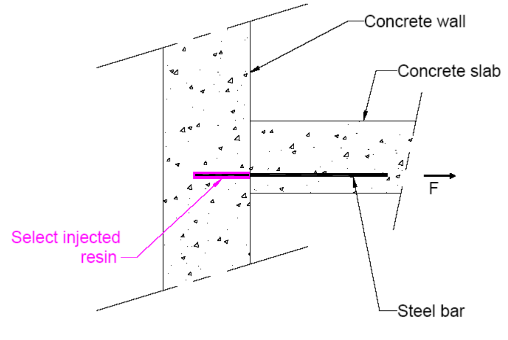
Concrete Cover (mm)	Fire Design Bond Resistance $f_{bd,FIRE}$ (N/mm²)					
	R30	**R60**	**R90**	**R120**	**R180**	**R240**
30						
40	0.8					
50	1.1					
60	1.5					
70	2.1	0.9				
80	2.9	1.2				
90	3.5	1.5	0.9			
100		1.8	1.1	0.8		
110		2.3	1.4	1.0		
120		2.8	1.6	1.2		
130		3.4	2.0	1.4	0.9	
140		3.5	2.3	1.6	1.0	
150			2.8	1.9	1.1	0.8
160			3.3	2.2	1.3	0.9
170			3.5	2.5	1.5	1.1
180				2.9	1.7	1.2
190				3.4	1.9	1.4
200				3.5	2.2	1.5
210					2.5	1.7
220					2.8	1.9
230					3.1	2.1
240					3.5	2.3
250						2.6
260						2.9
270						3.2
280						3.5
290						

The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.

Appendix 2.1:
Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 10 mm for diameters 8 and 10 mm

The table presents <u>design load resistances</u> for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **f_y=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 10 mm** and for diameters 8 and 10 mm.

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.



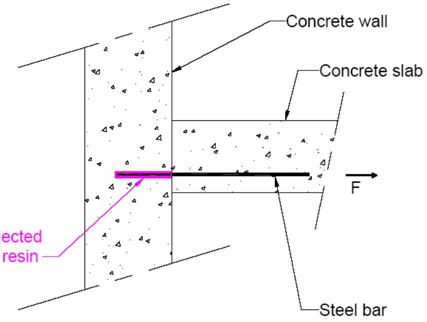
Concrete Cover = 10 mm		Fire Design Load Resistance $N_{Rd,Fire}$ (kN)					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
8	**100**	3.4	1.2	0.5	0.2	0.0	0.0
	140	6.9	4.1	2.2	1.4	0.5	0.0
	180	10.3	7.6	5.4	3.8	1.9	1.0
	220	13.8	11.1	8.9	7.2	4.3	2.7
	260	16.8	14.5	12.4	10.7	7.6	5.2
	290		16.8	15.0	13.3	10.3	7.8
	320			16.8	15.9	12.9	10.4
	330				16.8	13.7	11.3
	370					16.8	14.7
	400						16.8
10	**110**	5.3	2.2	1.0	0.5	0.0	0.0
	150	9.6	6.2	3.6	2.4	0.9	0.3
	190	14.0	10.6	7.9	5.8	3.0	1.7
	230	18.3	14.9	12.2	10.1	6.3	4.0
	270	22.7	19.2	16.5	14.4	10.6	7.6
	310	26.2	23.6	20.9	18.8	15.0	11.9
	340		26.2	24.1	22.0	18.2	15.2
	360			26.2	24.2	20.4	17.3
	380				26.2	22.6	19.5
	420					26.2	23.8
	450						26.2

Calculations are carried out taking the minimal concrete cover . Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.

Appendix 2.2:
Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 20 mm for diameters 8, 10, 12, 14, 16 and 20 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **f_y=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 20 mm** and for diameters 8, 10, 12, 14, 16 and 20 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.



Concrete Cover = 20 mm		Fire Design Load Resistance $N_{Rd,fire}$ (kN)					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
8	**100**	3.8	1.3	0.5	0.2	0.0	0.0
	140	7.2	4.3	2.3	1.5	0.7	0.2
	180	10.7	7.8	5.6	3.9	2.1	1.3
	220	14.2	11.2	9.1	7.4	4.6	2.9
	250	16.8	13.8	11.7	10.0	7.1	4.8
	290		16.8	15.1	13.5	10.6	8.1
	310			16.8	15.2	12.3	9.8
	330				16.8	14.0	11.6
	370					16.8	15.0
	390						16.8
10	**110**	5.8	2.4	1.1	0.6	0.0	0.0
	150	10.1	6.5	3.8	2.5	1.2	0.5
	190	14.5	10.8	8.1	6.0	3.3	2.0
	230	18.8	15.1	12.4	10.3	6.7	4.4
	300	26.2	22.7	20.0	17.9	14.3	11.2
	340		26.2	24.3	22.2	18.6	15.6
	360			26.2	24.4	20.8	17.7
	380				26.2	23.0	19.9
	410					26.2	23.1
	440						26.2

The table continues on the next page.

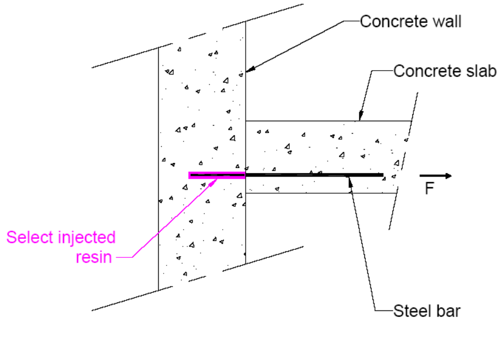
Concrete Cover = 20 mm		Fire Design Load Resistance $N_{Rd,fire}$ (kN)					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
12	**140**	10.9	6.5	3.5	2.3	1.0	0.3
	200	18.7	14.3	11.0	8.5	4.8	3.0
	260	26.5	22.1	18.8	16.3	12.0	8.3
	320	34.3	29.9	26.6	24.1	19.8	16.1
	350	37.7	33.8	30.5	28.0	23.7	20.0
	390		37.7	35.7	33.2	28.9	25.2
	410			37.7	35.8	31.5	27.8
	430				37.7	34.1	30.4
	460					37.7	34.3
	490						37.7
14	**160**	15.7	10.6	6.7	4.4	2.3	1.1
	220	24.8	19.7	15.8	12.9	8.0	5.1
	280	33.9	28.8	24.9	22.0	17.0	12.7
	340	43.0	37.9	34.1	31.1	26.1	21.8
	400	51.3	47.0	43.2	40.2	35.2	30.9
	430		51.3	47.7	44.8	39.7	35.4
	460			51.3	49.3	44.3	40.0
	480				51.3	47.3	43.0
	510					51.3	47.6
	540						51.3
16	**180**	21.4	15.5	11.2	7.8	4.3	2.5
	240	31.8	25.9	21.6	18.2	12.5	8.2
	300	42.2	36.3	32.0	28.6	22.9	18.0
	360	52.6	46.8	42.4	39.0	33.3	28.4
	450	67.0	62.4	58.0	54.6	48.9	44.0
	480		67.0	63.2	59.8	54.1	49.2
	510			67.0	65.1	59.3	54.4
	530				67.0	62.8	57.8
	560					67.0	63.0
	590						67.0
20	**220**	35.5	28.1	22.6	18.5	11.4	7.3
	280	48.5	41.1	35.6	31.5	24.3	18.1
	340	61.5	54.1	48.6	44.5	37.3	31.1
	400	74.5	67.1	61.7	57.5	50.3	44.1
	460	87.5	80.1	74.7	70.5	63.3	57.1
	540	104.7	97.5	92.0	87.8	80.6	74.5
	580		104.7	100.7	96.5	89.3	83.1
	600			104.7	100.8	93.6	87.5
	620				104.7	98.0	91.8
	660					104.7	100.5
	680						104.7

Calculations are carried out taking the minimal concrete cover. Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.

Appendix 2.3:
Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 30 mm for diameters 8, 10, 12, 14, 16, 20, 25 and 28 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **f_y=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 30 mm** and for diameters 8, 10, 12, 14, 16, 20, 25 and 28 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.



Concrete Cover = 30 mm		Fire Design Load Resistance $N_{Rd,fire}$ (kN)					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
8	**100**	4.1	1.6	0.8	0.2	0.0	0.0
	140	7.5	4.7	2.7	1.6	0.7	0.2
	180	11.0	8.2	6.0	4.1	2.2	1.3
	220	14.5	11.7	9.5	7.6	4.7	3.0
	250	16.8	14.3	12.1	10.2	7.3	4.9
	280		16.8	14.7	12.8	9.9	7.4
	310			16.8	15.4	12.5	10.0
	330				16.8	14.2	11.7
	360					16.8	14.3
	390						16.8
10	**110**	6.2	2.8	1.4	0.6	0.0	0.0
	150	10.5	7.0	4.3	2.6	1.3	0.5
	190	14.9	11.3	8.6	6.2	3.4	2.1
	230	19.2	15.7	12.9	10.5	6.9	4.5
	300	26.2	23.2	20.5	18.1	14.5	11.4
	330		26.2	23.7	21.4	17.8	14.7
	360			26.2	24.6	21.0	17.9
	380				26.2	23.2	20.1
	410					26.2	23.3
	440						26.2

The table continues on the next page.

Concrete Cover = 30 mm		Fire Design Load Resistance $N_{Rd,fire}$ (kN)					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
12	**140**	11.3	7.1	4.0	2.4	1.1	0.3
	200	19.1	14.9	11.6	8.7	5.0	3.1
	260	26.9	22.7	19.4	16.5	12.2	8.5
	320	34.7	30.5	27.2	24.3	20.0	16.3
	350	37.7	34.4	31.1	28.2	23.9	20.2
	380		37.7	35.0	32.2	27.8	24.1
	410			37.7	36.1	31.7	28.0
	430				37.7	34.3	30.6
	460					37.7	34.5
	490						37.7
14	**160**	16.2	11.3	7.4	4.6	2.4	1.2
	220	25.3	20.4	16.5	13.2	8.2	5.3
	280	34.5	29.5	25.6	22.3	17.3	12.9
	340	43.6	38.6	34.8	31.4	26.4	22.0
	400	51.3	47.7	43.9	40.5	35.5	31.2
	430		51.3	48.4	45.1	40.0	35.7
	450			51.3	48.1	43.1	38.7
	480				51.3	47.6	43.3
	510					51.3	47.8
	540						51.3
16	**180**	22.0	16.4	12.0	8.2	4.4	2.6
	240	32.4	26.8	22.4	18.6	12.8	8.4
	300	42.8	37.2	32.8	29.0	23.2	18.3
	360	53.2	47.6	43.2	39.4	33.6	28.7
	440	67.0	61.5	57.1	53.3	47.5	42.5
	480		67.0	64.0	60.2	54.4	49.5
	500			67.0	63.7	57.9	52.9
	520				67.0	61.4	56.4
	560					67.0	63.3
	590						67.0
20	**220**	36.2	29.1	23.6	18.9	11.8	7.6
	280	49.2	42.1	36.6	31.9	24.7	18.5
	340	62.2	55.1	49.6	44.9	37.7	31.5
	400	75.2	68.2	62.7	57.9	50.7	44.5
	460	88.2	81.2	75.7	70.9	63.7	57.5
	540	104.7	98.5	93.0	88.3	81.0	74.9
	570		104.7	99.5	94.8	87.5	81.4
	600			104.7	101.3	94.0	87.9
	620				104.7	98.4	92.2
	650					104.7	98.7
	680						104.7

The table continues on the next page.

Concrete Cover = 30 mm		**Fire Design Load Resistance $N_{Rd,fire}$ (kN)**					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
25	**280**	61.5	52.7	45.8	39.9	30.8	23.1
	340	77.8	68.9	62.1	56.1	47.1	39.4
	400	94.0	85.2	78.3	72.4	63.4	55.6
	460	110.3	101.4	94.6	88.7	79.6	71.9
	520	126.6	117.7	110.8	104.9	95.9	88.1
	580	142.8	134.0	127.1	121.2	112.1	104.4
	660	163.6	155.6	148.8	142.9	133.8	126.1
	690		163.6	156.9	151.0	141.9	134.2
	720			163.6	159.1	150.1	142.3
	740				163.6	155.5	147.8
	780					163.6	158.6
	800						163.6
28	**310**	78.0	68.1	60.4	53.8	43.6	35.0
	370	96.2	86.3	78.6	72.0	61.9	53.2
	430	114.4	104.5	96.8	90.2	80.1	71.4
	490	132.6	122.7	115.0	108.4	98.3	89.6
	550	150.8	140.9	133.2	126.6	116.5	107.8
	610	169.1	159.1	151.4	144.8	134.7	126.0
	670	187.3	177.4	169.7	163.0	152.9	144.2
	730	205.3	195.6	187.9	181.2	171.1	162.5
	770		205.3	200.0	193.4	183.2	174.6
	790			205.3	199.4	189.3	180.7
	810				205.3	195.4	186.7
	850					205.3	198.9
	880						205.3

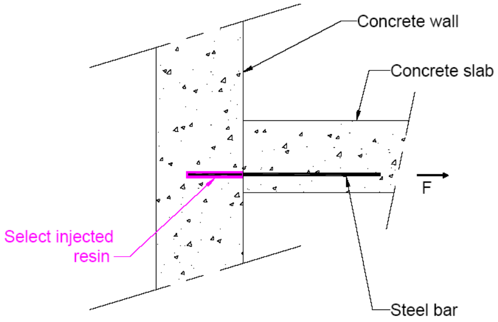
Calculations are carried out taking the minimal concrete cover. Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.

Appendix 2.4:

Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 40 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **f_y=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 40 mm** and for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.



Concrete Cover = 40 mm		Fire Design Load Resistance $N_{Rd,fire}$ (kN)					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
8	**100**	4.9	1.8	0.8	0.4	0.0	0.0
	140	8.4	5.0	2.9	1.9	0.7	0.2
	180	11.9	8.5	6.2	4.5	2.3	1.3
	220	15.4	11.9	9.7	8.0	4.9	3.1
	240	16.8	13.7	11.4	9.7	6.6	4.3
	280		16.8	14.9	13.2	10.1	7.6
	310			16.8	15.8	12.7	10.2
	330				16.8	14.4	11.9
	360					16.8	14.5
	390						16.8
10	**110**	7.3	3.1	1.5	0.9	0.0	0.0
	150	11.6	7.3	4.5	3.0	1.3	0.6
	190	15.9	11.7	8.9	6.7	3.5	2.1
	230	20.3	16.0	13.2	11.0	7.2	4.6
	290	26.2	22.5	19.7	17.5	13.7	10.5
	330		26.2	24.0	21.9	18.0	14.9
	350			26.2	24.0	20.2	17.0
	370				26.2	22.3	19.2
	410					26.2	23.6
	440						26.2

The table continues on the next page.

Concrete Cover = 40 mm		**Fire Design Load Resistance $N_{Rd,fire}$ (kN)**					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
12	**140**	12.6	7.5	4.3	2.8	1.1	0.3
	200	20.4	15.3	11.9	9.3	5.2	3.2
	260	28.2	23.1	19.7	17.1	12.5	8.8
	320	36.0	30.9	27.6	25.0	20.3	16.6
	340	37.7	33.5	30.2	27.6	22.9	19.2
	380		37.7	35.4	32.8	28.1	24.4
	400			37.7	35.4	30.7	27.0
	420				37.7	33.3	29.6
	460					37.7	34.8
	490						37.7
14	**160**	17.8	11.8	7.9	5.2	2.5	1.2
	220	26.9	20.9	17.0	13.9	8.5	5.5
	280	36.0	30.0	26.1	23.0	17.6	13.2
	340	45.1	39.1	35.2	32.1	26.7	22.4
	390	51.3	46.7	42.8	39.7	34.3	29.9
	430		51.3	48.8	45.8	40.4	36.0
	450			51.3	48.8	43.4	39.0
	470				51.3	46.4	42.1
	510					51.3	48.1
	540						51.3
16	**180**	23.8	16.9	12.5	9.0	4.6	2.7
	240	34.2	27.3	22.9	19.4	13.2	8.7
	300	44.6	37.7	33.3	29.8	23.6	18.6
	360	55.0	48.2	43.7	40.2	34.0	29.0
	430	67.0	60.3	55.8	52.3	46.1	41.2
	470		67.0	62.7	59.3	53.1	48.1
	500			67.0	64.5	58.3	53.3
	520				67.0	61.7	56.8
	560					67.0	63.7
	580						67.0
20	**220**	38.4	29.8	24.2	19.9	12.2	7.8
	300	55.7	47.2	41.6	37.3	29.5	23.3
	380	73.1	64.5	58.9	54.6	46.8	40.6
	460	90.4	81.9	76.3	71.9	64.2	57.9
	530	104.7	97.0	91.4	87.1	79.3	73.1
	570		104.7	100.1	95.8	88.0	81.8
	600			104.7	102.3	94.5	88.3
	620				104.7	98.9	92.6
	650					104.7	99.1
	680						104.7

The table continues on the next page.

Concrete Cover = 40 mm		**Fire Design Load Resistance $N_{Rd,fire}$ (kN)**					
Diameter (mm)	**Length lv (mm)**	**R30**	**R60**	**R90**	**R120**	**R180**	**R240**
25	**280**	64.2	53.6	46.6	41.1	31.4	23.7
	370	88.6	77.9	70.9	65.5	55.8	48.0
	460	113.0	102.3	95.3	89.9	80.2	72.4
	550	137.4	126.7	119.7	114.3	104.6	96.8
	650	163.6	153.8	146.8	141.4	131.7	123.9
	690		163.6	157.7	152.2	142.5	134.7
	720			163.6	160.4	150.7	142.9
	740				163.6	156.1	148.3
	770					163.6	156.4
	800						163.6
28	**310**	81.1	69.1	61.3	55.2	44.3	35.6
	370	99.3	87.3	79.5	73.4	62.5	53.8
	430	117.5	105.5	97.7	91.6	80.7	72.0
	490	135.7	123.7	115.9	109.8	98.9	90.2
	550	153.9	141.9	134.1	128.0	117.2	108.4
	610	172.1	160.1	152.3	146.2	135.4	126.6
	670	190.3	178.3	170.5	164.4	153.6	144.8
	720	205.3	193.5	185.7	179.6	168.7	160.0
	760		205.3	197.8	191.8	180.9	172.2
	790			205.3	200.9	190.0	181.3
	810				205.3	196.1	187.3
	850					205.3	199.5
	870						205.3
32	**350**	106.5	92.8	83.9	76.9	64.5	54.6
	410	127.3	113.6	104.7	97.8	85.3	75.4
	470	148.1	134.5	125.5	118.6	106.1	96.2
	530	168.9	155.3	146.3	139.4	127.0	117.0
	590	189.7	176.1	167.1	160.2	147.8	137.8
	650	210.6	196.9	187.9	181.0	168.6	158.6
	710	231.4	217.7	208.7	201.8	189.4	179.4
	820	268.1	255.8	246.9	240.0	227.5	217.6
	860		268.1	260.8	253.8	241.4	231.4
	890			268.1	264.2	251.8	241.8
	910				268.1	258.7	248.8
	940					268.1	259.2
	970						268.1

Calculations are carried out taking the minimal concrete cover. Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.